# Coupling stiffness and strength analysis for bulb hydropower station concrete foundation and turbine brackets

DONG Yun-feng<sup>1, a</sup>, JI Jin-li <sup>1, a</sup>, QU Xing-tian<sup>2, b</sup> and LI Zhi-he<sup>3, c</sup>

<sup>1</sup>Jilin Jianzhu University, Changchun, China

<sup>2</sup>Jilin University, Changchun, China

<sup>3</sup> Heerbin mechanical-electricity limited responsibility company, Harbin, China <sup>a</sup>247358698@qq.com, <sup>b</sup>quxt@jlu.edu.cn, <sup>c</sup>1294535402@qq.com

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**Abstract:** The paper applied finite element method to analysis and calculate the stress of bulb hydropower station concrete foundation and the deformation of turbine pipe base in all load case. The analysis results indicate the main piece reciprocal infection extent and supplies the theoretical basis for safety design of the project and the equipment.

#### Introduction

Bulb hydropower station concrete foundation bears the effect of electromagnetic force, gravity and hydraulic conditions caused by hydro-generator and its working. Getting to hold of these applied forces are the keys to design reasonable concrete foundations. Based on the running case of the hydropower station, we analyzed a variety of operating conditions which can be seen in Table 1.

Table 1: The basic parameters of the unit

basic parameter name	basic parameters	basic parameter name	basic parameters
maximum head	10.21m	turbine bearing load	53.5t
pressure head	15m	bubble head weight	26t
rated capacity	18000kw	weight of the stator	112t
rated speed	75r/min	bottom ring weight	11t
thrust forward water	297.66t	unilateral magnetic force	25.8t
water reverse thrust	297.66t	half of the short circuit	250t
		magnetic force	
upthrust	314t	electromagnetic torque	2550.6kN.m
		rating	
generator bearing load	102.6tt		

## FEM MODEL(The finite element calculation model)

By using ansys modeling, the shell 63 is selected as all the plate element models to analysis the turbine pipe base. The finite element calculation model is shown in Fig. 1 (a), bulb hydropower station concrete (b) the 1/4 of (a) foundation based on genera [1].

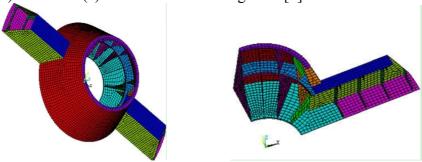


Fig. 1 Finite element calculating model

#### Boundary and working conditions of the loads

The loads under various working conditions are gathered in Table. 2 [2], which can be seen as follows:

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Table 7:	The	summary	ot v	arions	loads
raure 2.	1110	Summer y	OI V	arious	Todas

Working n	orm run	- two-ph	ase half of t	he no water	water wat	er water		
condition	away	short-	magnetic o			filling stop		
	circuit	short		no-load		8 1		
		circuit		unning	,			
Static load and								
bending	2/	$\sqrt{}$	$\sqrt{}$	$\sqrt{}$	$\sqrt{}$	2/	2	2/
moment of the	٧	V	V	V	V	V	V	V
bearing								
Rated speed	,		,					,
dynamic load	$\sqrt{}$		$\sqrt{}$	$\sqrt{}$			$\sqrt{}$	$\sqrt{}$
bearing								
Runaway speed		,						
dynamic load		V						
bearing								
Use of body	$\sqrt{}$	$\sqrt{}$	$\sqrt{}$	$\sqrt{}$	$\sqrt{}$	$\checkmark$	$\sqrt{}$	$\sqrt{}$
weight Bubble head								
weight of the	$\sqrt{}$	$\sqrt{}$	$\sqrt{}$	$\sqrt{}$	$\sqrt{}$	2/	ما	2/
stator	٧	V	٧	V	V	V	V	٧
Rated speed								
dynamic load	$\sqrt{}$			$\sqrt{}$				
bearing	•			•				
Runaway speed								
dynamic load			$\checkmark$	$\sqrt{}$				
bearing								
Use of body								
weight								

### The instance of calculating processes

This section shows an instance on working condition of water filling stop instantly to analyze the experience processes, and as many as eight working conditions were considered. In order to master the extend influence of the concrete foundation, we separately analyzed two conditions on the rigidity and infinite rigidity.

#### 3.1 The stress and strain of the calculating model without considering concrete foundation

Loading conditions (see Table 1). The stress distribution diagram is shown in Fig. 2, the maximum Von-mises stress is 242.296 MPa, and the maximum stress point of the structure is point H. The deformation distribution is shown in Fig. 3, the maximum comprehensive deformation is 3.725mm, and the maximum deformation point of the structure is HH.

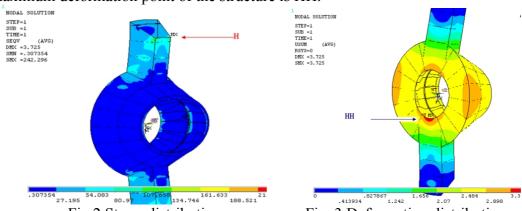


Fig.2 Stress distribution

Fig. 3 Deformation distribution

#### 3.2 The stress and strain of the calculating model concluding concrete foundation

According to the result of section 3.1, the maximum stress appeared in the combinations between tube constellation and concrete foundation. Two reasons were used to explain the exceeded stress, one is the weaken rigidity of tube constellation, another one is the equivalent stiffness of concrete foundation boundary. This section shows the calculation results according to the concrete foundation model and accurately analyzes the combinations. Fig. 4 shows the Calculation model.

Loading conditions (see Table 2). Elastic modulus and poisson ratio of concrete foundation were given in accordance with the building code [3-4].

The stress distribution is shown in Fig. 5. The maximum Von-mises stress is 163.091MPa, and the maximum stress located in point I.

The deformation distribution is shown in Fig. 6. The maximum comprehensive deformation is 4.915mm, and the maximum deformation located in point II.

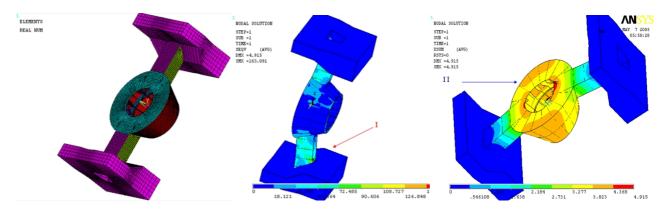


Fig.4 Calculation model

Fig. 5 Stress distribution

Fig.6 Deformation distribution

## The calculation results

The calculation results of the maximum stress, safety factors and the position summary under various working conditions are listed in Table 3.

Table 3: The calculation results of the maximum stress under various conditions

			safety factor criter		rding qualified	311411111111111111111111111111111111111
condition	n working	(MPa)	asses-sment to	the fatigue condition	or not life asses-	
			sment			
1	rated	81.456	2.88	$\leq 0.67\sigma_s$		qualified
2	feather	78.658	2.99	$\leq \sigma_s$		qualified
3	two-phase short circuit	91.601	2.57	fatigue life	fatigue life	qualified
4	Half of the magnetic short circuit	>235		fatigue life	more than 40 years	qualified
5	No water outage	33.889	6.93	$\leq 0.67\sigma_s$		qualified
6	Water filling stop	94.148	2.50	$\leq 0.67\sigma_s$		qualified
7	Water filling no-load running	75.945	3.09	$\leq 0.67\sigma_s$		qualified
8	Water filling stop instantly	163.091	1.44	$\leq \sigma_s$		qualified

The calculation results of the maximum deformation and the position summary under various working conditions are listed in Table 4.

Table 4 The calculation results of the maximum deformation under various conditions

working conditio n	name of working condition	maximum deformation (mm)
1	rated	1.344
2	feather	1.346
3	two-phase short-circuit	1.388
4	half of the magnetic short circuit	10.487
5	no water outage	0.45
6	Water filling stop	1.177
7	water filling no-load running	0.882
8	water filling stop instantly	4.915

The calculation results of the maximum deformation under directions of the rated conditions and the location are listed in Table 5.

Table 5 The calculation results of the maximum deformation under the rated conditions

computational condition	direction	maximum deformation (mm)
The magnetic force and buoyancy in the same direction	opposite direction with gravity	0.34
	direction perpendicular with gravity	0.41
	Axis direction	1.354
The magnetic force and	opposite direction with gravity	0.34
buoyancy in the direction perpendicular	direction perpendicular with gravity	0.46
	Axis direction	1.332

#### **Conclusions**

On the normal operation condition, the maximum comprehensive stress of the tube is less than two-thirds of the material yield limit, which meets the safety standard requirements. Under the working condition that the runaway stops and water is filling, the maximum comprehensive stress of the tube type is less than the material yield limit, which meets the safety standard requirements. At the same time, on the condition of two phase short circuit and magnetic short circuit of half the cases, the tube allows the impact of more than 40 times in the number. According to the annual inspection, the tube is provided to safely run more than 40 years. Due to the maximum comprehensive stress appears in the junction of concrete and tube type water filling stop instantly, the stiffness rigid of the concrete foundation is an important factor which should be considered into the calculation.

#### References

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