Numerical simulation analysis of surrounding rock displacement of double shield tunnel

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Keywords: Numerical simulation; Fluid-solid coupling; Surrounding rock displacement **Abstract.** Tunnel engineering is an important part of the development and utilization of underground space especially in the city. To alleviate traffic pressure, improve the efficiency of traffic greatly. This paper will use the numerical simulation software FLAC3D, to simulate the Subway tunnel excavation. Analysis of whether considering fluid solid coupling under two conditions of surrounding rock displacement distribution and the relationship between the settlement and the position of the nodes. The results show that, the tunnel of surrounding rock vault settlement is closely related to the node location, The closer from tunnel excavation area, the final settlement amount is large.

Introduction

With the rapid growth of population, The shortage of resources lead to the survival of the human resources overwhelmed, The development and utilization of underground space has become the focus of the world to explore and research. In this paper, using software FLAC3D to simulate the construction site to study the distribution of displacement field of surrounding rock^[1-3]. Using finite difference program FLAC-3D and comparison of measured data, verifying of the finite difference numerical simulation method of the reliability and the calculation results^[4-6].

Establishment of calculation model and calculation parameters

Rock density is 1800kg/m^3 , volume modulus is 1.47e8Pa, cohesion is 5.0e4Pa, the tensile strength of 1.0e4Pa, the shear modulus of elasticity is 5.6e7Pa, the friction angle is 20 degrees. Tunnel lining structure using C30 concrete, the density is 2500kg/m^3 , the bulk modulus is 16.7 Mpa, the modulus of shear is 12.5 Mpa, osmotic coefficient is $3.0 \times 10\text{-}4 \text{cm/s}$, porosity is 0.42%. Tunnel diameter is 6.0 m, lining thickness is 0.3 m, inside diameter is 5.4 m, the depth is 10 m. the model size is $72 \text{m} \times 36 \text{$

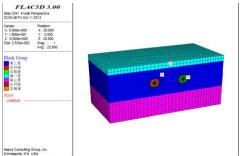


Fig.1 Dimensional model of double shield tunnel

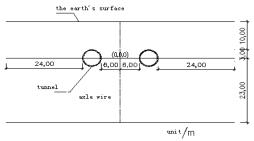
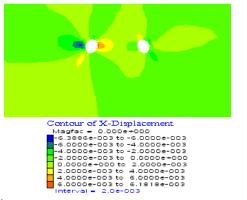


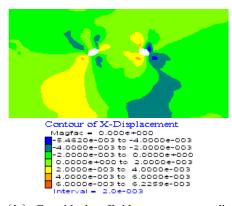
Fig.2 Y = 0m section layout of metro shield tunnel

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The horizontal displacement field of surrounding rock

Based on the above numerical model we will to study surrounding rock displacement field distribution., the analysis of two kinds working conditions of whether to consider the fluid solid coupling, as follows:





(a) Not considering fluid-structure coupling

(b) Considering fluid-structure coupling

Fig.3 Surrounding rock horizontal displacement nephogram after double tunnel excavation

Figure 2 (a), (b) respectively the Surrounding rock horizontal displacement nephogram under the conditions of whether to consider the fluid solid coupling. Among them, ribtht is positive, left is negative, the unit is mm. By figure (a) shows that, without considering the condition of fluid solid coupling effect the maximum displacement distribution concentrated in near the excavation area, the maximum horizontal displacement value is 6.39mm, by the side wall in the tunnel to both sides of the expansion; By figure (a) shows that, considering the effect of fluid solid coupling the maximum horizontal displacement value is 6.23mm, appeared in the both sides of the tunnel, the displacement of surrounding rock is the convergence to the side after tunnel excavation. It can be seen from the above, whether or not to consider the fluid solid coupling effect, the horizontal displacement are relatively small, mainly due to the tunnel surrounding rock radial convergence, the seepage make little effect to the field of surrounding rock displacement.

The vertical displacement distribution of surrounding rock

In the numerical simulation, monitoring the vertical displacement of tunnel, select vault node of the left and right hole, the monitoring node number as shown in the table 1.

Tab.1 Surrounding rock displacement monitoring node location of tunnel and soft

Monitoring	Location	Node number
node umber	/m	
1	-9,18,3	18
2	-8,18,6	22
3	9,18,3	32
4	8,18,6	34
5	-9,18,-3	37
6	9,18,-3	41
7	-8,18,-6	43
8	8,18,-6	44

Leading-out the monitor node displacement data of numerical simulation by using the Hist command, deal with the information by Data processing software of Excel, Setting every 100 steps to monitor

the data, considering fluid-structure coupling, obtained the vertical settlement of vault with two conditions of whether considering the effect of fluid-structure coupling.

Fig.4 is the double tunnel arch monitoring node settlement graph, of which not considering fluid-structure coupling. It can be seen from the figure, surrounding rock deformation is obvious in the range of tunnel excavation, the influence of vertical displacement of surrounding rock has reached the surface, the vertical displacement of the vault surrounding rock is negative, indicating that the tunnel vault settlement, and produces a downward displacement of surrounding rock in a certain region above the vault, leading to the subsidence of ground surface above the tunnel, the maximum settlement of the vault has reached 11.8mm;

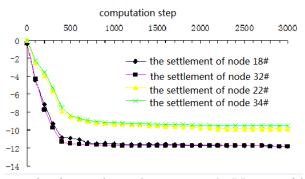


Fig.4 Double tunnel arch monitoring node settlement graph (Not considering fluid-structure coupling)

Fig.5 is the Double tunnel arch monitoring node settlement graph ,of which considering fluid-structure coupling. It can be seen from the figure, under the condition of considering the fluid solid coupling effect, the vertical deformation of the monitoring nodes is gradually increases, along with the increase of calculation time step, the final calculated settlement of the monitoring nodes (18# and 32# node)of the left and right tunnel is about 21.4mm, the final settlement of monitoring nodes (32# node)of the tunnel right hole are slightly less than 21.4mm, surrounding rock disturbance caused by the excavation of left tunnel, so that the final settlement of the monitoring nodes is about 21.4mm;

when left and right tunnel excavation of about $5\,\text{m}$, the value of arch crown settlement can reach the maximum, then it tends to be stable; The amount of settlement is associated with the node position, the nearer to the tunnel excavation zone , the larger the final settlement. Farther away from the tunnel excavation zone , the smaller the final settlement of the other nodes (34#, 22# node) .

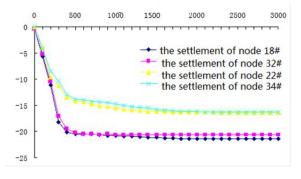


Fig.5 Double tunnel arch monitoring node settlement graph (considering fluid-structure coupling)

Under the condition of considering the fluid solid coupling effect, final settlement range and settlement of vault nodes of monitoring were significantly increased compared with the condition of not considering the fluid solid coupling effect, the consolidation settlement caused by the decrease of pore water pressure in the funnel - shaped zone; the change law of settlement of the vault monitoring nodes are consistent with the not considering fluid-structure coupling effect.

Summary

- (1)Under the condition of considering fluid-structure coupling and not considering fluid-structure coupling, the amount of settlement is associated with the node position, the nearer to the tunnel excavation zone, the larger the final settlement. The final settlement amount of the monitoring nodes of right tunnel are slightly smaller than the left one, because of surrounding rock disturbance caused by the first excavation of left tunnel.
- (2)Because of the tunnel excavation, the pressure of the pore water in the zone of funnel is reduced, so that consolidation settlement. The final settlement range and settlement of vault nodes of monitoring were significantly increased compared with the condition of not considering the fluid solid coupling effect.

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