3 types of vehicle-bridge coupling vibration analysis of model comparison

^aChen xu-yong, ^bSong teng-teng, ^cLuo lu

^aWuhan Institute of Technology, Wuhan, China ^bWuhan Institute of Technology, Wuhan, China cWuhan Institute of Technology, Wuhan, China

Key words: Vehicle-bridge coupling vibration; Vehicle model; Vibration

Abstract: Compares two 1/4 car models, two 1/4 model (changes the spring stiffness) and 1/2 car models three vehicles at the same speed in the displacement effect. Comparison: when the vehicle speed is less than 130km/h, and displacement response curves in the three models are calculated when a highway bridge coupling vibration analysis of vehicle is two one-fourth model can be used to simulate.

Introduction

Vehicle-bridge coupled vibration attracts many researchers' attention as early as 100 years ago. The current study has simplified the vehicle and bridge model in it to make for a simpler solution.

In recent years, many Chinese researchers are trying to use numerical method to solve coupling problems. The document^[4] builds overall 3D model to make the interaction between vehicle and bridge as a whole, thus establishing differential equations, with the kelp of numerical methods, the dynamic response is obtained with respect to simple-supported girder and continuous beam in highway. The document^[5-6] makes a model of vehicle bridge coupling of simple-supported beam bridge employing d'alembert's principle and uses numerical methods to solve the problem. Step-by-step integral is used to calculate the coupling system consisting of mass with uniform variable speed and a simple supported beam. The document^[8] acquires the power series solution to Willis equation. The document^[9] derives the vibration equation by ignoring moving constant loads of mass vehicle and thus gets the exact solution. The document^[10] uses concentrated mass as the simplified bridge model and adds 2D multi-axis trailer load.

To investigate the influence of vehicle model on vehicle-bridge coupling, three kinds of models:two quarter-car models(spring stiffness changed) and half-car models with different velocity, are used to analyze displacement response. The overall laws are analyze in the first place which followed the analysis of difference of models.

Model

one-fourth car models

Figure 1 is vehicle model, the spring-damper-mass system is for simulating vehicles. Where m_b is vehicle mass. m_t is the frame and wheel mass; k_a is vertical stiffness; c_a is vertical damping;

 k_b is vertical stiffness; c_b is vertical damping; v is the speed when vehicles go cross the bridge.

The vibration equation of vehicle and bridge are as follows:

$$m_{t} \frac{d^{2} y_{t}}{dt^{2}} + m_{b} \frac{d^{2} y_{b}}{dt^{2}} + c_{a} \left(\frac{dy_{t}}{dt} + \frac{dy_{w}}{dt}\right) + k_{a} (y_{t} + w) = 0,$$

$$m_{b} \frac{d^{2} y_{b}}{dt^{2}} + c_{b} \left(\frac{dy_{b}}{dt} + \frac{dy_{t}}{dt}\right) + k_{b} (y_{b} - y_{t}) = 0.$$

Supposing $w(x,t) = \sum_{n} X_{n}(x)T_{n}(t)$ by using variable separation method. Using simple

supported beam as boundary condition and presuming that $X_n(x) = \sin(npx/l)$. According the orthogonality of principal mode, the vibration differential equation under loads is as follows:

$$\ddot{T}_{n}(t) + w_{n}^{2}T_{n} = \frac{2(m_{t} + m_{b})g}{ml} \sin \frac{pvt}{l} + \\ \ddot{T}_{n}(t) + \frac{2m_{t}}{ml} \frac{dy_{t}}{dt^{2}} \sin \frac{npvt}{l} + \frac{2m_{b}}{ml} \frac{d^{2}y_{b}}{dt^{2}} \sin \frac{npvt}{l} \\ \text{With the help of variable separation method, suppose } w(x,t) = \sum_{n} X_{n}(x)T_{n}(t) \text{ .By}$$

simply-supported beams with boundary condition, let $X_n(x) = \sin(npx/l)$, Again according to the principal mode of orthogonality, get the vibration differential equation of the bridge under the action of load as follows:

$$\ddot{T}_{n}(t) + w_{n}^{2}T_{n} = \frac{2(m_{t} + m_{b})g}{ml}\sin\frac{pvt}{l} + \frac{2m_{t}}{ml}\frac{dy_{t}}{dt^{2}}\sin\frac{npvt}{l} + \frac{2m_{b}}{ml}\frac{d^{2}y_{b}}{dt^{2}}\sin\frac{npvt}{l},$$

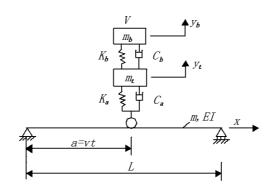


Fig. 1. One-fourth model under the effect of simply supported beam

Two quarter-car models will be used. The vibration equations for vehicle and bridge are taken into consideration separately, he the results are in superposition. The other one is the same.

Half-car models

Figure 2 is vehicle model, the spring-damper-mass system is for simulation of vehicles. Where m_b is vehicle mass. I_b is the rigidity of vehicle body; m_t is the mass of wheels and structures; k_a is vertical stiffness; c_a is vertical damping; k_b is vertical stiffness; c_b is vertical damping; v is the speed when vehicles go cross the bridge(constant value). The vibration equations are as follows^[6]:

 $\frac{dT_n(t)}{dt^2} + w_n^2 T_n(t) = \frac{2p_1(t)}{ml} \sin \frac{np(vt-a)}{l} d_1(t) + \frac{2p_2(t)}{ml} \sin \frac{npvt}{l} d_2(t)$ Which:

Which:

$$d_{1}(t) = \begin{cases} 1, & \frac{a}{v} \le t \le \frac{l+a}{v}; \\ 0, & \text{else} \end{cases}, \qquad d_{2}(t) = \begin{cases} 1, & 0 \le t \le \frac{l}{v}; \\ 0, & \text{else} \end{cases},$$

Analysis of load of the vehicle system:

$$p_{1}(t) = m_{a1}g + \frac{m_{b}}{2}g + m_{a1}y_{t1} + \frac{m_{s}}{2}y_{b} - \frac{I_{b}}{a}q$$

$$p_{2}(t) = m_{t2}g + \frac{m_{b}}{2}g + m_{t2}y_{t2} + \frac{m_{b}}{2}y_{b} - \frac{I_{b}}{a}q$$

" **7** "]

Analysis of load of the bogie:

$$m_{t1}\ddot{y}_{t1} + c_{a1}(\dot{y}_{t1} + \dot{w}_{1}) + k_{a1}(y_{t1} + w_{1}) + c_{b1}(\dot{y}_{t1} - \dot{y}_{b} + \frac{\dot{a}}{2}\dot{q}) + k_{b1}(y_{t1} - y_{b} + \frac{\dot{a}}{2}q) = 0$$

$$m_{t2}\ddot{y}_{t2} + c_{a2}(\dot{y}_{t2} + \dot{w}_{2}) + k_{a2}(y_{t2} + w_{2}) + c_{b2}(\dot{y}_{t2} - \dot{y}_{b} - \frac{\dot{a}}{2}\dot{q}) + k_{b2}(y_{t2} - y_{b} - \frac{\dot{a}}{2}q) = 0$$
 Which:

$$w_{1} = \sum_{n} X_{n}(vt - a)T_{n}(t); \qquad w_{2} = \sum_{n} X_{n}(vt)T_{n}(t)$$

Analysis of load of the body:

$$m_{b} \ddot{y}_{b} + c_{b1} (\dot{y}_{b} - \dot{y}_{t1} - \frac{a}{2} \dot{q}) + k_{b1} (y_{b} - y_{t1} - \frac{a}{2} q) - c_{b2} (\dot{y}_{t2} - \dot{y}_{b} - \frac{a}{2} \dot{q}) - k_{b2} (y_{t2} - y_{b} - \frac{a}{2} q) = 0$$

$$I_{b} \ddot{q} - \frac{a}{2} \left[c_{b1} \left(\dot{y}_{b} - \dot{y}_{t1} - \frac{a}{2} \dot{q} \right) + k_{b1} \left(y_{b} - y_{t1} - \frac{a}{2} q \right) \right] + \frac{a}{2} \left[c_{b2} \left(\dot{y}_{b} - \dot{y}_{t2} - \frac{a}{2} \dot{q} \right) + k_{b2} \left(y_{b} - y_{t2} - \frac{a}{2} q \right) \right] = 0.$$

$$y_{1} + \frac{b}{k_{b1}} \left[c_{b1} + \frac{b}{k_{b2}} \right] + \frac{a}{2} \left[c_{b2} \left(\dot{y}_{b} - \dot{y}_{t2} - \frac{a}{2} \dot{q} \right) + k_{b2} \left(y_{b} - y_{t2} - \frac{a}{2} \dot{q} \right) \right] = 0.$$

Fig. 2.Half-car model under the effect of simply supported beam

Results

Bridge data in document^[11] and vehicle data in document^[12] are used, the length of simple supported beam is L=32m. Mass per unit length $m = 5.41 \times 10^{3} kg/m$. flexural rigidity $EI = 3.5 \times 10^{10} N \bullet m^{2}$. Following are relevant data of quarter-car model: $m_{t1} = m_{t2} = 4330 kg$; $m_{b} = 19250 kg$; a = 8.4m; $k_{a1} = k_{a2} = 4.28 \times 10^{6} N/m$; $k_{b1} = k_{b2} = 2.535 \times 10^{6} N/m$; $c_{a1} = c_{a2} = 9.8 \times 10^{4} kg/s$; $c_{b1} = c_{b2} = 1.96 \times 10^{5} kg/s \cdot 1/2$ car models: $c_{a1} = c_{a2} = 9.8 \times 10^{4} kg/s$; $m_{b1} = m_{t2} = 4330 kg$; $m_{b1} = 38500 kg$; $m_{t1} = m_{t2} = 4330 kg$; $I_{b} = 2.446 \times 10^{6} kg \bullet m^{2}$; a = 8.4m;

$$k_{a1} = k_{a2} = 4.28 \times 10^6 \, N/m$$
; $k_{b1} = k_{b2} = 2.535 \times 10^6 \, N/m$; $c_{b1} = c_{b2} = 1.96 \times 10^5 \, kg/s$

Figure $3 \sim$ Figure 6 compare 3 kinds of vehicles vertical displacement at mid-point of beam at same speed. Figure 7 shows the largest displacement of the 3 kinds of models on the bridge under different speed. The comparison of curves in the figure shows that the 3 kinds of models could embody coupling response.

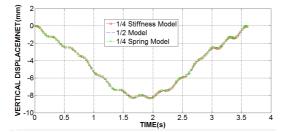


Fig. 3. Vertical displacement at mid-point of beam at speed 40km/h

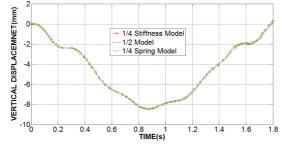


Fig. 4. Vertical displacement at mid-point of beam at speed 80km/h

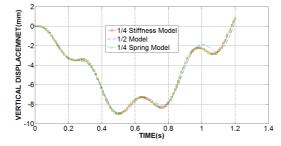


Fig. 5. Vertical displacement at mid-point of beam at speed 120km/h

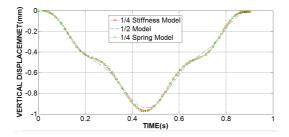


Fig. 6. Vertical displacement at mid-point of beam at speed 160km/h

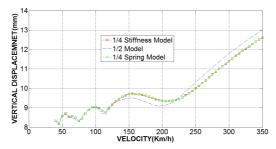


Fig. 7. The largest displacement on the bridge under different speed

General laws could be arrived at based on above figures:

(1) They all give good expression to the vibration response.

(2) Higher speed will make a slower displacement response of mid-span.

(3) Response Maximum displacement in bridge when vehicle appear in mid-span.

(4) When speed is under 200km/h, response of maximum displacement bridge do not take on a linear relationship.

The difference between vehicle models are as follows:

(1) The difference between 1/4 spring model of rigidity and 1/4 spring model is the change of rigidity. The displacement response in mid-span of bridge fits well with each other. Spring stiffness has no impact on vehicle-bridge coupling.

(2) When speed is under 130km/h, the discrepancy of maximum displacement in 1/4 vehicle model and 1/2 vehicle model is small.

(3) When speed is under 220km/h, the maximum displacement in 1/2 vehicle model is smaller than that 1/2 vehicle model. The vehicle-bridge coupling is more sensitive to moment of inertia.

(4) When speed is more than 220km/h, the maximum displacement in 1/2 vehicle model is greater than that 1/2 vehicle model.

(5) Generally speaking, the vehicle speed is no more than 120/h, The vehicle-bridge coupling can be realized using 1/4 model in highway bridge.

Conclusions

To study the response of factor of vehicle model to the vehicle-bridge coupling, the displacement of mid-span of 3 kinds of models are caculated. Comparison shows that the 3 kinds of models are in good agreement with vibration response of vehicle-bridge coupling. With the add of rotational inertia, the agreement will be better yet make the analysis complicated. When velosity is under 130km/h, the 3 kinds of models are in good agreement with each other. Quarter-car models can be used to analyse vibration of vehicle-bridge coupling in highway bridge.

References:

[1] MichaltsosG,Sophianopoulos D, KounadisA N. The effect of a moving mass and other parameters on the dynamic response of a simply supported beam[J]. Journal of Sound and Vibration, 1996; 191(3): 357-362.

[2] Cai Y, Chen S S, Rote D M, et al. Vehicle/guideway interraction for high speed vehicles on a flexible guideway[J]. Journal of sound and Vibration, 1994; 175(5): 625-646.

[3] Ma Kunqun, Cao Xueqin. Train lateral vibration analysis of through continuous beam bridge with high Pier [J]. Shanghai: Shanghai tiedao University, 1993,15 (1): 9-15.

[4] Wang Yuanfeng, Xu Shijie. Study on dynamic response of bridge under the vehicle space [J]. China Journal of highway and transport. 2000,13 (4): 38-41.

[5] Xiaoxinbiao, Shen Huoming. Bridge simulation System under moving load[J]. Vibration and impact, 2005,24 (1): 121-123.

[6] Shen Huoming, xiaoxinbiao. Solution of a numerical method for vehicle-bridge coupling vibration[J]. Journal of Southwest Jiaotong University, 2003,38 (6): 658-662.

[7] Peng Xian, Yin Xinfeng, Fang Zhi.Variable speed vehicle-bridge coupling vibration and TMD control [J]. Journal of Hunan University(Natural Sciences), 2006,26 (5): 19-21,37.

[8] Stokes G G.Discussions of a differential equation related to the breaking of railway bridges[J].Trans Cambridge PhilSoc,1896,8:12-16.

[9] Kolousek V,Flyba L.Civil engineering structures subjected to dynamic load[M].Bratislava:SVTL,1967:54-58.

[10] Andersen L,Nielsen S R K,Iwankiewicz R.Vehicle movingalong an infinite beam with random surface irregularities on a Kelvin foundation[J]. Journal of Applied Mechanics,2002:69-75.

[11] Xue Ding-Yu, Chen Yangquan. MATLAB/Simulink-based simulation technology and application[M]. Beijing: Tsinghua University Press, 2002:180-200.

[12] Zhai Wanming. Vehicle-track coupling Dynamics[M]. Beijing: China railway Publishing House, 2001:360-370.