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Study on Plane-section Assumption of New Composite T-shaped Concrete-filled Steel Tubular Columns

J Huang^{1, 2}, S Q Dai^{1, *}, J Y Xie¹, C Wang¹

¹School of Civil Engineering and Architecture, Wuhan University of Technology, Wuhan 430070, China

²Design & Research Institute, Wuhan University of Technology, Wuhan 430070, China E-mail: 1779375740@qq.com

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Abstract. The linear regularity of the strain distribution about new composite T-shaped concrete-filled steel tubular columns on the multiple sections was analyzed by ABAQUS. The plane-section assumption of new composite T-shaped concrete-filled steel tubular columns was studied under the axial loading and lateral loading, and the impact of the load angle has been analyzed. The analysis results show that the sharp change will appear on section of T-shaped concrete-filled steel tubular columns under axial loading and lateral loading, but it basically satisfies the plane-section assumption; T-shaped concrete-filled steel tubular columns basically conform to the plane-section assumption under different load angles (except the 90°load angle), the influence of load angle on the plane-section assumption is not obvious.

Introduction

This research presents a new type of composite T-shaped steel pipe concrete column, which based on analyzing the study of special-shaped concrete-filled steel tubular columns at home and abroad. As shown in Figure 1, this composite column is a rectangular steel pipe directly welding with a U-shaped steel plate, and usually can be divided into T-shaped, L-shaped and Ten-shaped. On the basis of the research on the axial compression performance and seismic performance [1-9], the plane-section assumption of T steel tube concrete column is studied by ABAQUS in this paper. And the study result will lay the foundation for calculation it bending bearing capacity and nonlinear analysis.

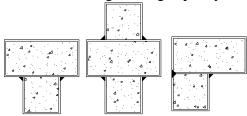


Figure 1. Schematic diagram of special-shaped concrete-filled steel tubular columns

Specimen Parameters

For frame columns with fixed ends and lateral displacement at one end, assuming that it's reasonable for beam column inflection point located at the mid span, so a simplified cantilever column model is selected, and the specimen model and section size as shown in figure 2 and figure 3. This paper selects C30 concrete, Q235 steel, and the steel tube wall thickness is 6 mm, the column length is 3m.



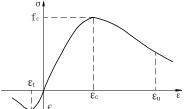
Figure 2. The cantilever column model Figure 3. Sectional dimensions of test pieces



Finite element model

Constitutive relation of concrete. As shown in figure 4, the model considering the influence of the constraint effect coefficient, and the peak strain and the descending section of the stress-strain relationship of the uniaxial concrete are corrected.

Constitutive relation of steel. Idealization elastic-plastic model is adopted, as shown in Figure 5. Under the multi-axial stress states, the Von Mises yield criterion proposed by Von Mises is used for the yield criterion of steel.



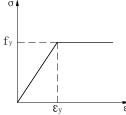


Figure 4. Constitutive relation of concrete Figure 5. The constitutive relation of steel

Interface model of steel and concrete. The normal contact between steel pipe and concrete adopts

the penalty function ('hard contact'), and the tangential contact adopts the coulomb friction model. **Element type selection and meshing.** The four-node shell element (S4R) and the eight-node three-dimensional solid element (C3D8R) are respectively selected as the steel and concrete element types. The mesh size of the steel tube and the concrete are all defined as 0.05m. The concrete is divided by the structured grid division technique. The steel tube is meshed by the sweeping mesh technology, as shown in Figure 6.

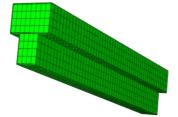


Figure 6. Finite element model of T-shaped concrete-filled steel tubular columns

Constraint conditions and loading. The X, Y, Z three directions of displacement and rotation in column bottom should be constrained. The axial load on the column end is applied to the upper end of the column in the form of a uniform load, and the axial force is transmitted to the column. The top of the column is within 200 mm of the column as the loading zone and the horizontal load is loaded as a force on the side. The vertical load is also loaded on the top surface as surface force.

Plane-section assumption under axial loading

Set the axial compression ratio of the column to 0.4, and applied axial loading. For a more accurate analysis of plane-section assumption for T-shaped concrete-filled steel tubular columns, the strain distributions on several sections were studied. The strain distribution of the central axis at the Zm section is the strain distribution at the X-direction central axis from the bottom Zm section, as shown in Figure 2.

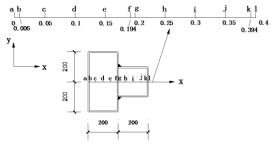


Figure 7. Diagram of measuring point position



As shown in Figure 7: a, b, c, d ... l is twelve measuring points on the X central axis, 0.4 represents that the most edge distance from the cross-section X direction is 0m, 0.006m ... 0.394m, 0.4m. The axial strain of twelve points under different stress states was analyzed by ABAQUS. In this paper, the strain is taken as the vertical axis, and the distance from the measuring point to the edge of the cross section X direction is taken as the horizontal axis, the distribution of strain scattergrams at different cross sections is plotted, as shown in Figure 8 to Figure 13.

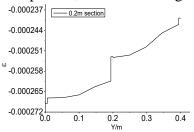


Figure 8. The central axis strain distribution of X direction in 0.2m section

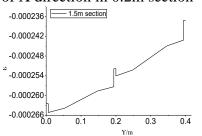


Figure 10. The central axis strain distribution of X direction in 1.5m section

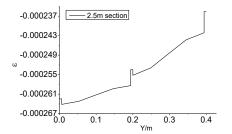


Figure 12. The central axis strain distribution of X direction in 2.5m section

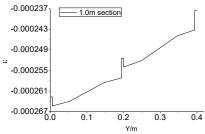


Figure 9. The central axis strain distribution of X direction in 1.0m section

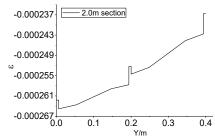


Figure 11. The central axis strain distribution of X direction in 2.0m section

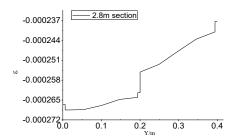


Figure 13. The central axis strain distribution of X direction in 2.8m section

As can be seen from the above strain profile, the strain distribution of T-shaped concrete-filled steel tubular columns will produce abrupt change. The site of the mutation occurs where the pipe contacts with the concrete (the middle and both ends of the cross section in the X direction), and this mutation is sometimes very large and has an adverse effect on the assumption of plane section. The reason why the mutation is mainly due to that the elastic modulus of steel is one magnitude order higher compared with that of concrete, and the deformation of steel and concrete under load is inconsistent. But the strain distribution is approximated as a straight line at some sections. The strain scatters on different cross sections have been fitted by Origin, and obtains the coefficient of determination ($\frac{\cdot}{R}$), as shown in Table 1.

Table 1. Fitting decomposition coefficient of strain in different sections under axial loading

Section	0.1m	0.2m	0.5m	1.0m	1.5m	2.0m	2.7m	2.8m
Section								
R		0.94349	0.94743	0.93072	0.92495	0.91618	0.90178	

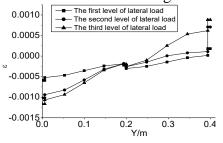
In the analysis of the data in the table, it can be known that under the action of the axial load, the fitting coefficient of the strain scatter fitting in the section is basically over 0.90 from $0.2m \sim 2.7m$, which shows that it satisfies the linear distribution. Therefore, the T-shaped concrete-filled steel



tubular columns accord with the assumption of plane section under axial load, and the strain of section at H/15-H/11.1 is tribute linearly.

Plane-section assumption under lateral load

Set the axial compression ratio of the column to 0.4, and applied axial loading, then applied to the lateral load, loaded to the steel yield (yield strength 235Mpa), this load mode makes the component in bending and torsion state. It is calculated that when the axial compression ratio is 0.3, the lateral load is added to the yield of 92 kN, steel began to yield. When the lateral load is applied, it is divided into four load steps. The first load step applies the axial load, the second load step applies 35kN lateral load as the first lateral load, and the third load step applies 70kN lateral load as the secondary lateral load and applies 90kN lateral load is applied as the third lateral load in the fourth load step. The results of strain distribution are shown in Figure 14 to Figure 19.



0.0008 The first level of lateral load The third level of lateral load 0.0004 0.0000 **ω** -0.0004 -0.0008 -0.0012 0.0 0.2 0.3 0.4 V/m

Figure 14. The central axis strain distribution of X direction in 0.2m section

The first level of lateral load 0.0005 The second level of lateral lo 0.0002 -0.000 -0.0004-0.0010 0.0

Figure 15. The central axis strain distribution of X direction in 0.5m section

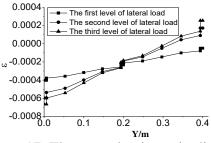
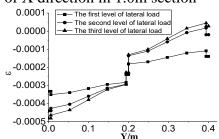


Figure 16. The central axis strain distribution of X direction in 1.0m section



0.2

0.3

Figure 17. The central axis strain distribution of X direction in 1.5m section

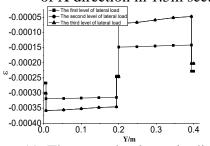


Figure 18. The central axis strain distribution of X direction in 2.0m section

Figure 19. The central axis strain distribution of X direction in 2.5m section

As can be seen from the above strain profile, the strain distribution of T-shaped concrete-filled steel tubular columns will still have the mutation occurs where the pipe contacts with the concrete, and the size of this mutation varies with the location of the cross-section changes., in which the mutations above sections 2.2m were significantly larger. The strain distribution is approximated as a straight line at sections 1.0m to sections 1.8m. But at sections 2.2m to sections 2.5m, the strain deviation from the straight line in the section is more serious, obviously not consistent with the linear distribution. And the determination coefficients of strain scatter fitting at different cross sections have been analyzed, as shown in Table 2.



racios. Them g decomposition eventient of strain in different sections								
Different levels of lateral load	0.1m section	0.2m section	0.5m section	1.0m section	1.5m section	2.0m section	2.1m section	2.2m section
First level	0.90504	0.92836	0.93333	0.96320	0.99203	0.90235		
Second level		0.91733	0.92601	0.96297	0.99091	0.94425	0.90130	
Third level		0.98642	0.91819	0.94713	0.98863	0.96065	0.92754	

Table 2. Fitting decomposition coefficient of strain in different sections

By analyzing the Table2, we can know that the T-shaped concrete-filled steel tubular column under the lateral load in accordance with the assumption of plane cross-section. Under the condition of first lateral loads, the strain is linearly distributed in the range of $0.1\text{m}\sim2.0\text{m}$ (1/30~1/1.5) H, and in the condition of second lateral loads, the strain is linearly distributed in the range of $0.2\text{m}\sim2.1\text{m}$ (1/15~1/1.43) H. The strain is linearly distributed in the range of $0.2\text{m}\sim2.1\text{m}$ (1/15~1/1.43) H under the condition of third lateral loads. Therefore, the variation of the lateral load has little effect on the assumption of the column plane cross-section, and under different lateral loadings, the cross-section of the strain is linearly distributed in the range of $0.2\text{m}\sim2.1\text{m}$ (1/15~1/1.43) H.

The effect of loading angle

The cross-section of the T-shaped columns of concrete-filled steel tubes is symmetrical about the i-i axis, and the loading angle is studied only when the loading angle is 0° , 45° , 90° , 135° , and 180° . To ensure that the force F does not change, by changing the size of F_x , F_y to change the loading angle. The strain distribution under the loading angle is shown in Figure 20 to Figure 25. It can be seen from the above analysis that the strain on the cross section will produce a sudden change in the contact area between the steel tube and the concrete under different loading angles. And in order to further study the influence of loading angle, the fitting coefficient of different strain points have been analyzed, as shown in table 3.

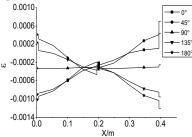


Figure 20. Strain distribution in 0.2m section

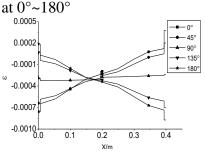


Figure 22. Strain distribution in 1.0m section at $0^{\circ} \sim 180^{\circ}$

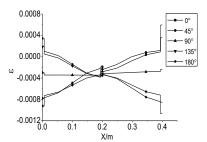


Figure 21. Strain distribution in 0.5m section

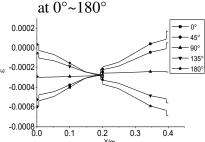
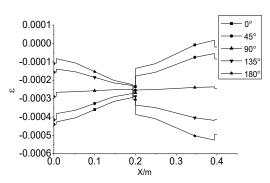


Figure 23. Strain distribution in 1.5m section at $0^{\circ} \sim 180^{\circ}$





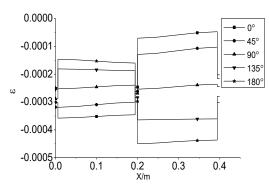


Figure 24. Strain distribution in 2.0m section at 0°~180°

Figure 25. Strain distribution in 2.5m section at $0^{\circ} \sim 180^{\circ}$

Table3. The fitting coefficient of different strain points under different loading angles

Tubies. The fitting everificant of different strain points under different founding ungles								
Section	0°	45°	90°	135°	180°			
0.1m	_	0.93377	_	0.97104	0.92911			
0.2m	0.91733	0.98987		0.97961	0.97437			
0.5m	0.92601	0.96237		0.97030	0.95660			
1.0m	0.96297	0.97716		0.97709	0.97602			
1.5m	0.99091	0.99096		0.99094	0.99232			
1.9m	0.97007	0.97532		0.96501	0.96066			
2.0m	0.94425	0.95605		0.93729	0.93265			
2.1m		0.92334						
2.2m					_			

Analysis of Table 3 shows that the section strain scatter fitting coefficients between 0.2m to 2.0m section are more than 0.90 under different loading angles (except 90° loading angle), indicating that the strain distribution in line with the linear distribution, and the action of T-shaped concrete-filled steel tubular column in line with the plane-section assumption (except 90° loading angle). The section strain does not accord with the linear distribution under 90° loading angle, so it does not conform to the plane-section assumption. The reason for this situation is because at 90° loading angle, the lateral load is applied only F_y without the application of F_x , the bending is mainly bending along the neutral axis of bending, and other angles are generated under the action of bending around the neutral axis. However, in the actual project will not have a purely 90 degree loading angle, so the assumption of the plane section is still established.

Conclusion

In this paper, the plane section assumption of new composite T-shaped concrete-filled steel tubular columns under axial load and lateral load is studied, and by analysis the effect of loading angle, the follow conclusions can be drawn:

- (1) Under the action of axial load, the T-shaped concrete-filled steel tubular columns are in accordance with the assumption of plane section. The strain distribution of the T-shaped concrete-filled steel tube is linearly distributed in the range of (1/15 to 1/11.1) H.
- (2) When the axial compression ratio is 0.3, the T-shaped concrete-filled steel tubular column under the lateral load is consistent with the assumption of plane cross-section, the strain distribution in the cross section is linearly distributed in the range of $0.2m \sim 2.0m (1/15 \sim 1/1.5)$.
- (3) When the axial compression ratio is 0.3, the T-shape concrete-filled steel tubular columns can meet the assumption of plane cross-section at different loading angles. The strain distribution conforms to the linear distribution range of $0.2m \sim 2.0m (1/15H \sim 1/1.5 H)$; the loading angle has little influence on the T-shaped concrete-filled steel tube.



Acknowledgements

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References

- [1] Lin Mingsen, Dai Shaobin, Liu Jixiong & Peng Zhong, 2012. Experimental study on seismic performance of end-plate connections of concrete-filled steel tube T-section column and steel beam [J]. Earthquake Engineering and Engineering Vibration, 2012, 32(2): 114-119.
- [2] Cao Bing, Dai Shaobin, Lin Mingsen, etc., 2012. Experimental study on seismic performance of connections between T-shaped concrete-filled steel column and steel beam [J]. Applied Mechanics and Materials, 2012, 204-208: 2455-2460.
- [3] Cao Bing, Dai Shaobin, Huang Jun, etc., 2012. Research on load-displacement skeleton curve model of L-shaped concrete-filled steel tubular column [J]. Advances in Civil Engineering and Building Materials, 2012, 514-518: 655-659.
- [4] Cao Bing, Dai Shaobin, Huang Jun, 2013. Orthogonal analysis on T-shaped concrete-filled steel tubular column seismic performance based on finite element calculation results [J]. Advanced Materials Research. Vols. 671-674, pp 523-528, 2013.
- [5] Cao Bing, Dai Shaobin, Huang Jun, 2013. Research on load-displacement skeleton curve model of T-shaped concrete-filled steel tubular column [J]. Journal of Hefei University of Technology, 2013, 36 (10):1222-1226, 1258.
- [6] Liu JiXiong, Dai Shaobin, Peng Yao, etc., 2013. Experimental and numerical studies on the joint of special-shaped concrete-filled rectangular composite tubular column with H-shaped beam. Advanced Materials Research, 2013, 671-674: 417-423.
- [7] Cao Bing, 2013. Nonlinear finite element analysis on seismic performance of T-shaped steel tubular column. Wu Han: Wuhan University of Technology master degree thesis, 2013.
- [8]Lin Hong, Liu JiXiong, Liu Ming, etc., 2014. Comparison analysis of evaluation criterions for vibration serviceability induced by wind load [J]. Applied Mechanical and Materials, 2014, 578-579: 653-658.
- [9] Liu Jixiong, Dai Shaobin, Huo Kaicheng, etc., 2015. Study on earthquake behavior of top and seat angle joint of special-shaped concrete-filled rectangular composite tubular column with steel beam [J]. Journal of Sichuan university (engineering science edition), 2015, 47 (1): 128-137.