

Effect of Pipeline-Soil Interaction on Subsea Pipeline Lateral Buckling Analysis

Yuxiao Liu^{1,a,*}

¹Shandong Technology and Business University, Yantai, China

^a 63563298@qq.com

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Abstract. Finite element method is used for post-buckling analysis of pipeline usually. In order to study effect of interaction between pipeline and seabed, FEM of pipeline exposed on seabed is built based on ANSYS, interaction between pipeline and seabed is studied, as results show that critical buckling force is increased with lateral frictional coefficient increasing, axial frictional coefficient has little effect on critical buckling force; critical buckling force is reduced with yielding displacement increasing, which can be used for engineering design.

1. Introduction

In recent years, there has been a rapid increase in the number of subsea pipelines transporting high pressure and high temperature hydrocarbons. Pipelines operating at high temperatures and pressures above ambient will tend to expand, due to thermal and pressure loading^[1]. When the axial compressive force is large enough that the pipeline will buckle globally. For the buried pipeline, upheaval buckling will be triggered; and for the pipeline exposed on even seabed, lateral buckling will be observed^[2].

Hobbs studied the thermal buckling of pipeline induced by the transportation of high temperature fluid^[3-4]. He used perfect pipe and the model of small deflection beam-column on rigid foundation to analyze the thermal buckling behaviors of beam vertical mode and beam lateral mode. Taylor and Tran^[5] studied upheaval buckling of subsea pipeline experimentally and theoretically. Several researchers have investigated the effects of initial imperfections, but the curved pipe was assumed to be stress-free when initially deformed. Croll^[6] studied upheaval thermal buckling of subsea pipeline based on a simplified model, critical buckling force was derived, but, the pre-buckling force was not given. Yuxiao Liu^[7] studied lateral buckling of imperfect pipeline, and critical axial forces in buckling segment and away from buckling segment as well as critical temperature are deduced.

However, the global buckling formula can only be used to calculate the pipeline critical buckling load, which can not be used for post-buckling analysis of pipeline deformation, strain and moment. In order to study effect of interaction between pipeline and seabed, FEM of pipeline exposed on seabed is built based on ANSYS, interaction between pipeline and seabed is studied, as results show that critical buckling force is increased with lateral frictional coefficient increasing, axial frictional coefficient has little effect on critical buckling force; critical buckling force is reduced with yielding displacement increasing.

2. Finite Element Model

2.1. Pipeline Model

ANSYS is used to study the lateral buckling of snaked-lay pipeline. The pipeline is modeled as PIPE20 elements with plastic capability. Which stress-strain constitutional relationship is shown in Fig.1. The expression of the Ramberg-Osgood model is:

$$\epsilon_x = \frac{\sigma_x}{E_0} \left[1 + \frac{n}{1+r} \left(\frac{\sigma_x}{\sigma_y} \right)^r \right] \tag{1}$$

Where: ϵ_x and σ_x are the engineering strain and stress; E_0 is the initial Young's modulus; σ_y the yield stress of pipe material; n and r are the Ramberg-Osgood parameters.

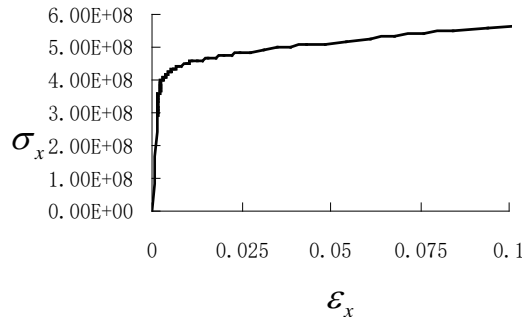


Fig.1 Stress-Strain Relationship For X65 Pipeline Steel.

2.2. Soil Model

The pipe-soil nonlinear interactions in the axial and lateral directions are simulated as the elastic perfectly-plastic soil springs. COMBIN39 spring element is selected. An axial spring and a lateral spring are connected to each pipe node. The seabed is simplified as flat one and spanning part of the pipeline is ignored. The pipeline is pinned in the vertical direction at each pipe node. The soil model is shown in Fig. 2, where F_L is the maximum axial and lateral soil forces per unit length of pipe, Δ_{L_y} is the yield displacements of soil springs in both directions. The relationship between yield force, friction coefficient and submerged weight along pipeline per unit can be expressed as:

$$F = \mu W \tag{2}$$

Where, F is yield force; μ is friction coefficient between seabed and pipeline; w is submerged weight along pipeline per unit.

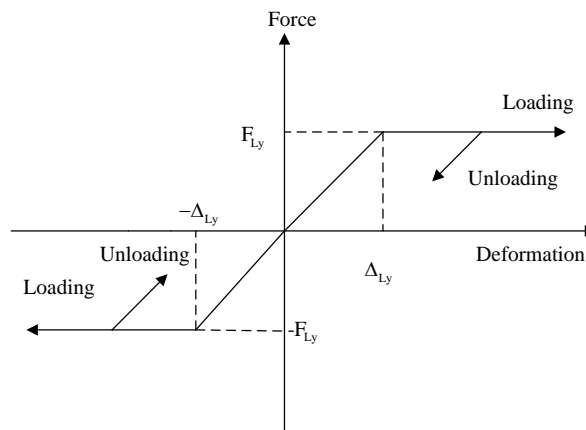


Fig.2 Model for Soil.

2.3. Pipeline-soil interaction model

Non-linear ideal elasto-plastic soil axial and lateral friction behavior was modeled using the COMBIN39 spring element [8-10], as Fig.3 shows. As only lateral buckling is of interest in this paper, the seabed was modeled as a flat seabed ignoring spanning parts of the pipeline. The pipeline was pinned in the vertical direction at each pipe node. FEM of pipeline is shown in Fig.4

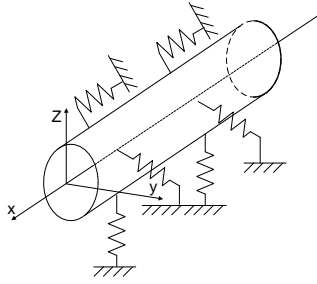


Fig.3. Pipeline-soil interaction model.

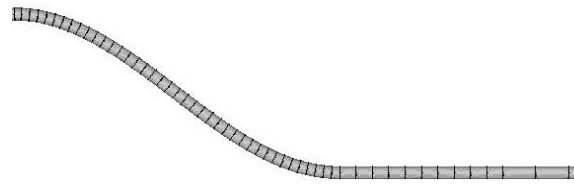


Fig. 4 Pipeline finite element model.

3. Pipeline-Soil Interaction

3.1. lateral frictional coefficient

Constant both pipeline and soil properties were used in the analyses. These properties are given in Table 1 together with the operating parameters.

Table 1 Parameters of pipeline.

| parameters | unit | value |
|-------------------------------|------|-----------------------|
| Pipe diameter | mm | 300 |
| Pipe thickness | mm | 14 |
| Thermal expansion coefficient | — | 11.7×10^{-6} |
| Pipeline submerged weight | N/m | 900 |
| SMYS/ MPa | MPa | 448 |
| SMTS/ MPa | MPa | 550 |
| Derating stress at 100°C | MPa | 25 |
| Lateral friction coefficient | — | 0.75 |
| Axial friction coefficient | — | 0.5 |
| Pipeline length | m | 2800 |
| Operating press | MPa | 20 |
| Operating temperature | °C | 95 |
| Ambient temperature | °C | 0 |

Fig.5 is variation of maximum lateral displacement with temperature under different axial frictional coefficient, as figure shows, critical temperature is increased with lateral coefficient increasing.

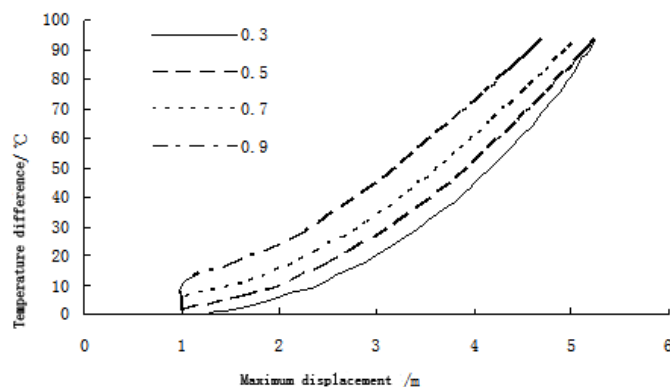


Fig. 5 Temperature vs. maximum lateral displacement for different lateral frictional coefficients.

3.2. Axial frictional coefficient

Fig.6 is variation of maximum lateral displacement with temperature under different axial frictional coefficient, as figure shows, critical temperature changed little as axial frictional coefficient changed.

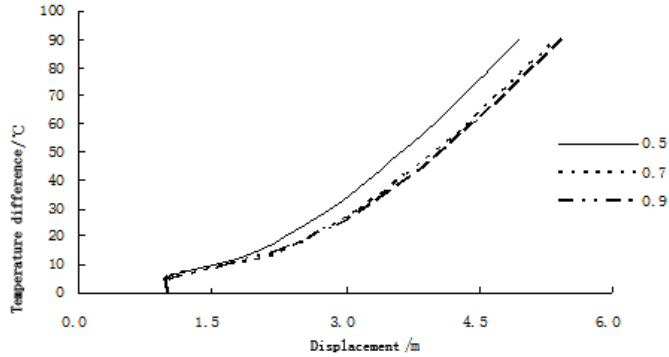


Fig. 6 Temperature vs. maximum lateral displacement for different axial frictional coefficients.

3.3. Soil yield displacement

Fig.7 is variation of maximum lateral displacement with temperature under different soil yield displacement, as figure shows, critical temperature is decreased with soil yield displacement increasing.

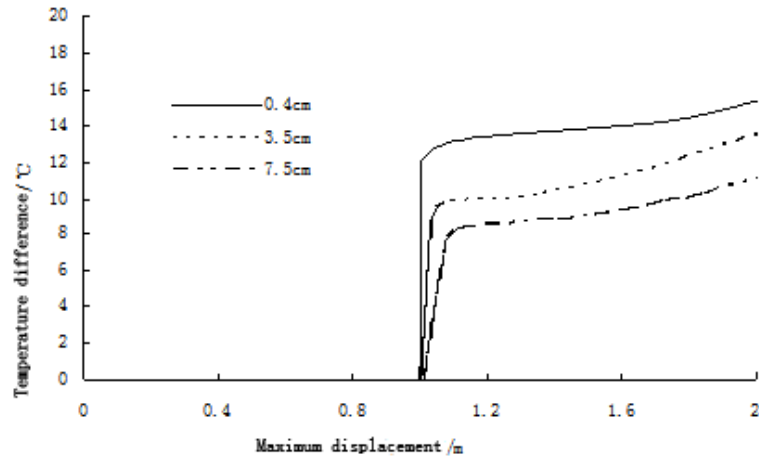


Fig. 7 Temperature vs. maximum lateral displacement for different axial frictional coefficients.

4. Conclusion

FEM of pipeline exposed on seabed is built based on ANSYS, interaction between pipeline and seabed is studied, as results show that critical buckling force is increased with lateral frictional coefficient increasing, axial frictional coefficient has little effect on critical buckling force; critical buckling force is reduced with yielding displacement increasing.

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